

# ACI 550.5M-18

An ACI Standard

## Code Requirements for the Design of Precast Concrete Diaphragms for Earthquake Motions (ACI 550.5M-18) and Commentary

Reported by Joint ACI-ASCE Committee 550



American Concrete Institute  
*Always advancing*



## **Code Requirements for the Design of Precast Concrete Diaphragms for Earthquake Motions (ACI 550.5M-18) and Commentary**

Copyright by the American Concrete Institute, Farmington Hills, MI. All rights reserved. This material may not be reproduced or copied, in whole or part, in any printed, mechanical, electronic, film, or other distribution and storage media, without the written consent of ACI.

The technical committees responsible for ACI committee reports and standards strive to avoid ambiguities, omissions, and errors in these documents. In spite of these efforts, the users of ACI documents occasionally find information or requirements that may be subject to more than one interpretation or may be incomplete or incorrect. Users who have suggestions for the improvement of ACI documents are requested to contact ACI via the errata website at <http://concrete.org/Publications/DocumentErrata.aspx>. Proper use of this document includes periodically checking for errata for the most up-to-date revisions.

ACI committee documents are intended for the use of individuals who are competent to evaluate the significance and limitations of its content and recommendations and who will accept responsibility for the application of the material it contains. Individuals who use this publication in any way assume all risk and accept total responsibility for the application and use of this information.

All information in this publication is provided “as is” without warranty of any kind, either express or implied, including but not limited to, the implied warranties of merchantability, fitness for a particular purpose or non-infringement.

ACI and its members disclaim liability for damages of any kind, including any special, indirect, incidental, or consequential damages, including without limitation, lost revenues or lost profits, which may result from the use of this publication.

It is the responsibility of the user of this document to establish health and safety practices appropriate to the specific circumstances involved with its use. ACI does not make any representations with regard to health and safety issues and the use of this document. The user must determine the applicability of all regulatory limitations before applying the document and must comply with all applicable laws and regulations, including but not limited to, United States Occupational Safety and Health Administration (OSHA) health and safety standards.

Participation by governmental representatives in the work of the American Concrete Institute and in the development of Institute standards does not constitute governmental endorsement of ACI or the standards that it develops.

Order information: ACI documents are available in print, by download, through electronic subscription, or reprint and may be obtained by contacting ACI.

Most ACI standards and committee reports are gathered together in the annually revised the ACI Collection of Concrete Codes, Specifications, and Practices.

**American Concrete Institute**  
**38800 Country Club Drive**  
**Farmington Hills, MI 48331**  
**Phone: +1.248.848.3700**  
**Fax: +1.248.848.3701**

[www.concrete.org](http://www.concrete.org)

# Code Requirements for the Design of Precast Concrete Diaphragms for Earthquake Motions (ACI 550.5M-18) and Commentary

An ACI Standard

Reported by Joint ACI-ASCE Committee 550

Larbi M. Sennour\*, Chair

Lance Osborne, Secretary

Suzanne Aultman  
Roger J. Becker\*  
Te-Lin Chung  
Ned M. Cleland\*  
Manuel Conde Fuentes  
Thomas J. D'Arcy\*  
William K. Doughty  
Semeh Ibrahim El Ashri

Alvin C. Ericson  
Mostafa Mohamed Gad Alla  
Harry A. Gleich  
Neil M. Hawkins†  
Augusto H. Holmberg  
L. S. Paul Johal  
Jason J. Krohn\*  
Emily B. Lorenz

Kenneth A. Luttrell  
Vilas S. Mujumdar  
Clay J. Naito\*  
Clifford R. Ohlweiler  
Victor F. Pizano-Thomen  
Charles L. Pizzano  
Jose I. Restrepo\*  
Sami H. Rizkalla

Mario E. Rodriguez\*  
Joseph C. Sanders\*  
James Schroder  
John F. Stanton  
P. Jeffrey Wang  
Cloyd E. Warnes  
Michael H. Weber

\*Diaphragm Subcommittee members who developed this code

†Diaphragm Subcommittee Chair

Special thanks to S. K. Ghosh and S. Nakaki for their contributions to this standard.

*This standard describes code requirements for the design of precast concrete diaphragms subject to earthquake motions where used under the design provisions of ASCE/SEI 7-16 Section 12.10.3 and ACI 318M. The response of precast concrete diaphragms under earthquake motions depends primarily on the strength, stiffness, and deformation capacities of the connectors and the reinforcement at joints between the precast concrete members. The seismic forces specified in ASCE/SEI 7 for the design of precast concrete diaphragms, their chords, and collectors in structures assigned to Seismic Design Category (SDC) C, D, E, or F are tied to force reduction factors specified in ASCE/SEI 7-16 Chapter 12, and to*

*the shear overstrength provided by the connections and the reinforcement at joints specified in ASCE/SEI 7-16 Chapter 14. The shear overstrength depends on the design methodology, elastic or ductile, used for the diaphragm and targets elastic response for the maximum considered earthquake for shear connections regardless of the design option selected. The design option that can be used depends on the assigned design category and on the span and aspect ratio of the diaphragm. The selection of the design option is associated with minimum requirements for the tensile deformation capacity of the connections and the reinforcement at joints.*

**Keywords:** connections; diaphragms; earthquake-resistant structures; precast concrete; seismic design.

ACI Committee Reports, Guides, and Commentaries are intended for guidance in planning, designing, executing, and inspecting construction. This document is intended for the use of individuals who are competent to evaluate the significance and limitations of its content and recommendations and who will accept responsibility for the application of the material it contains. The American Concrete Institute disclaims any and all responsibility for the stated principles. The Institute shall not be liable for any loss or damage arising therefrom.

Reference to this document shall not be made in contract documents. If items found in this document are desired by the Architect/Engineer to be a part of the contract documents, they shall be restated in mandatory language for incorporation by the Architect/Engineer.

ACI 550.5M-18 was adopted August 13, 2018, and published September 2018.

Copyright © 2018, American Concrete Institute.

All rights reserved including rights of reproduction and use in any form or by any means, including the making of copies by any photo process, or by electronic or mechanical device, printed, written, or oral, or recording for sound or visual reproduction or for use in any knowledge or retrieval system or device, unless permission in writing is obtained from the copyright proprietors.

## CONTENTS

### CHAPTER 1—GENERAL, p. 3

- 1.1—Introduction, p. 3
- 1.2—Scope, p. 4
- 1.3—Structural drawings, p. 4
- 1.4—Units, p. 5

### CHAPTER 2—NOTATION AND DEFINITIONS, p. 6

- 2.1—Notation, p. 6
- 2.2—Definitions, p. 6

### CHAPTER 3—REFERENCED STANDARDS, p. 8

### CHAPTER 4—GENERAL CONSIDERATIONS, p. 9

- 4.1—General design considerations, p. 9
- 4.2—Materials, p. 9
- 4.3—Minimum thickness, p. 9
- 4.4—Tolerances, p. 9

### CHAPTER 5—DESIGN FORCES, SEISMIC DEMAND LEVELS, AND ANALYSIS, p. 11

- 5.1—General, p. 11

- 5.2—Diaphragm seismic design force, p. 11
- 5.3—Diaphragm seismic demand level, p. 12
- 5.4—Diaphragm nominal shear strength, p. 13
- 5.5—Diaphragm modeling and analysis, p. 15

### CHAPTER 6—DIAPHRAGM DESIGN OPTIONS, p. 17

- 6.1—General, p. 17
- 6.2—Elastic design option, p. 20
- 6.3—Basic design option, p. 20
- 6.4—Reduced design option, p. 20

### CHAPTER 7—DIAPHRAGM CONNECTIONS AND REINFORCEMENT AT JOINTS, p. 21

- 7.1—General, p. 21
- 7.2—Connection classifications, p. 21
- 7.3—Deformed bar reinforcement, p. 22
- 7.4—Special inspection, p. 22

### R8—COMMENTARY REFERENCES AND ACRONYMS, p. 23

- R8.1—References, p. 23
- R8.2—Acronyms, p. 23



## CHAPTER 1—GENERAL

## R1—GENERAL

**1.1—Introduction**

**1.1.1** Consistent with **ACI 318M** requirements for analysis, this standard specifies expected performance and design requirements for precast concrete diaphragms subject to earthquake loading. This standard is meant to replace the design procedure for precast concrete diaphragms for structures assigned to Seismic Design Category (SDC) C, D, E, or F that was developed and accepted for use by ASCE/SEI 7-16 in Section 14.2.4. The procedure described herein and that of **ASCE/SEI 7-16** supplement the provisions of Chapter 18 of ACI 318M-14 and do not supplant them.

**1.1.2** The procedure described herein may also be used for precast concrete diaphragms in structures assigned to SDC B.

**R1.1—Introduction**

Precast concrete diaphragms are extensively used for parking structures and residential and commercial buildings. Those diaphragms frequently consist of large precast, prestressed concrete members such as double-tee (DT) or hollow-core (HC) members. Double-tee members are connected to one another through discrete mechanical connections or by reinforcement that crosses the joint between members. Industry practice is to use these DT diaphragms in an untopped condition in buildings assigned to SDC A and B, and in a topped condition in buildings assigned to SDC C, D, E, or F. Hollow-core members are primarily used in an untopped condition in buildings assigned to SDC A and B.

Design requirements for precast concrete diaphragms are covered by the general provisions of **ACI 318M**. However, unless a precast concrete diaphragm includes a topping that meets all the prescriptive requirements for diaphragms in Chapter 18 of ACI 318M-14, the precast concrete diaphragm cannot be designed directly using that chapter. For DT diaphragms made composite with a topping or without a topping, structural integrity and force transfer within the diaphragm are provided by the discrete web and chord connections that join the individual precast concrete members. If a precast concrete diaphragm made composite with a topping or without a topping is to provide a structural system with an earthquake loading performance equal to or exceeding that of a comparable cast-in-place concrete diaphragm, accurate knowledge of the strength, stiffness, and deformability of the individual connections used in the diaphragm is needed. Results from tests on individual connections in accordance with **ACI Standard 550.4M-18** are needed to obtain the information on stiffness, shear strength, tensile strength, and tensile deformation capacity required for the design of connections and reinforcement at joints for precast concrete diaphragms satisfying the lateral load performance requirements of this standard, Section 12.10.3 of **ASCE/SEI 7-16**, and Chapter 18 of ACI 318M-14.

Post-earthquake reconnaissance following the 1994 Northridge earthquake (**Iverson and Hawkins 1994**) revealed that when precast concrete diaphragms with topping of 75 mm or less were subjected to significant earthquake motions, the topping was likely to crack along the edges of the precast concrete members. Consequently, reinforcement crossing the edges was susceptible to damage and the degree of susceptibility increased as the aspect ratio for the diaphragm increased and as the larger dimension of the diaphragm between seismic-force-resisting vertical elements increased.

Those observations on the behavior of large precast concrete diaphragms resulted in a comprehensive research study (**Fleischman 2014**) to develop better design models for precast concrete diaphragms and comprehensive studies of the strength and deformation capacity of diaphragm connections. In the improved design methodology resulting from that research, the choice of connection type is tied to the tension deformations and shear overstrength needed in the diaphragm to achieve the required design performance.

## CODE

## COMMENTARY

Where connections with limited deformation capacity are used, the earthquake design forces need to be higher than for ductile connections. The choice of the appropriate overstrength that should be used in diaphragm design requires detailed knowledge of the strength and deformation capacities of the diaphragm connections for the differing combinations of force and deformation experienced by the connections.

**1.2—Scope**

**1.2.1** This standard shall apply to precast concrete diaphragms and collectors that are part of the seismic-force-resisting system in structures assigned to SDC C, D, E, or F. It is permissible to use this standard for the design of the same elements in structures assigned to SDC B.

**1.2.2** This standard shall apply to precast concrete diaphragms, including a) through c):

- a) Diaphragms that consist of a cast-in-place composite topping slab with a thickness of less than 75 mm on precast concrete members
- b) Diaphragms that comprise precast concrete members with end strips formed by either a cast-in-place composite topping or edge beams
- c) Diaphragms of interconnected precast concrete members without cast-in-place concrete topping.

**1.3—Structural drawings**

Structural drawings for precast concrete diaphragms shall show all features of the members into which the connectors or reinforcement at joints are cast that are essential to the intended seismic performance of the diaphragms and all details of the connections or reinforcement at joints essential for attainment of that intended performance.

Essential details shall include:

- a) The anchorage of the connectors and reinforcement at joints into the precast concrete member
- b) The procedures and materials by which the connection between connectors in adjacent members shall be made and the tolerances that are acceptable
- c) Supplemental reinforcement that shall be included in the precast concrete members to ensure that the performance of the member under earthquake loading does not materially affect the measured response of the connections between members
- d) The methods to be used to ensure composite action, as specified in the design, between topping and precast concrete member
- e) The connection of the collectors and the adjacent precast concrete members to the vertical elements of the seismic-force-resisting system
- f) The quality control and special inspection procedures governing placement of connectors and completion of connections

**R1.3—Structural drawings**

Reinforcement details in the vicinity of the connectors and the means and procedures by which the connections between the precast concrete members are completed affect the performance of the connection. Details should be specified completely, including tolerances, and fully documented on the structural drawings for the diaphragm.